A Finite Element approach for mesoscopic modelling of fracture

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2 Damage model

3 Applications

- Macroscopic behavior
- Young age concrete
- Scale effects

4 Conclusions

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Introduction : Scales of modelling

Scales of Modelling

- Structure > 10m
- Macroscopic $\approx 1 10n$
- Mesoscopic pprox 1 10cm
- Microscopic< 1 cm



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Introduction : Scales of modelling

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Introduction : Scales of modelling

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- Mesoscopic $\approx 1 10$ cm
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D Crack width

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Introduction : Scales of modelling

Scales of Modelling

- Structure > 10m
- Macroscopic $\approx 1 10n$
- Mesoscopic ≈ 1 10*cm*
- Microscopic< 1 cm</p>

Cement paste E/C = 0.45



from D.P. Bentz (NIST, CEMHYD3D)

Introduction : mesoscopic models

- Witmann 1988 : Application to drying of concrete
- Mounajed, Menou & La Borderie ≈ 2002 : Symphonie, concrete at high temperatures
- Implementation into Cast3M : 2007

Smooth method : 2010



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specifications

- 2D or 3D random generation
- granular compactness
- ITZ
- Shape of aggregates

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The smooth meshing method



Meshing methods

Exact method

- Each aggregate is meshed separately, then the matrix is meshed
- Doesn't work in case of small and large aggregates, particularly in 3D
- ITZ can be modelled (but which parameter ?)



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Meshing methods

Discrete method

- Aggregates properties are projected on the mesh elements
- Costless method, irregularities at the boundary
- Total volume of aggregates is badly modeled



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Meshing methods

Smooth method

- Aggregate properties are projected on the Gauss points
- Each Gauss point owns the material properties of paste or aggregate
- Costless method, unable to model ITZ



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Aggregate drawing

C. La Borderie & collaborators FE Meso Fracture

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Mesh samples



$$\Phi_{min} = 1 mm$$





C. La Borderie & collaborators FE Meso Fracture



$$\Phi_{min} = 2.5 mm$$

Damage model

Damage model

- Isotropic but unilateral (Fichant et al, 1999)
- Indirect effect of damage on en compression
- Softening in tension
 - Mazars' equivalent strain $\tilde{\varepsilon}$
 - mesh size h
 - parameters ft et Gf

•
$$\bar{\sigma}_{ij} = \frac{E}{1+\nu} \varepsilon_{ij} + \frac{E\nu}{(1+\nu)(1-2\nu)} \varepsilon_{kk} \delta_{ij}$$

• $\sigma_{ij} = (1 - d) \langle \bar{\sigma} \rangle_{ij}^{+} + (1 - d)^{\alpha_1} \langle \bar{\sigma} \rangle_{ij}^{-}$
• $d = 1 - \frac{f_t}{E\tilde{\varepsilon}} exp\left(\frac{hf_t}{G_f} \left(\frac{f_t}{E} - \tilde{\varepsilon}\right)\right)$

| | E(GPa) | ν | $f_t(MPa)$ | $G_f(J/m^2)$ | α_1 | | | |
|-------------------------------|--------|-----|------------|--------------|------------|--|--|--|
| Paste | 15 | 0.2 | 3 | 20 | 10 | | | |
| Aggregates | 60 | 0.2 | 6 | 60 | 30 | | | |
| Characteristics of components | | | | | | | | |

Characteristics of components

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Macroscopic behavior Young age concrete Scale effects

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Application to macroscopic behavior



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Uniaxial tension

Macroscopic behavior

Crack width

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Uniaxial tension



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Uniaxial compression

Comportement macroscopique

Ouverture de fissure

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Uniaxial Compression



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Effects of hydration on inner stresses

Hydration

- Thermo-activation (UIm)
- Hydration heat
- Endogenous shrinkage

 $\dot{\xi} = \tilde{A}(\xi) e^{-\frac{E_a}{RT}}$

- ξ hydration degree
- $\tilde{A}(\xi)$ Normalized affinity function
- *E_a* is the activation energy (*Jmol*⁻¹)
- *R* is the constant of perfect gas (8.314*Jmol*⁻¹*K*⁻¹)
- T is the temperature in kelvin

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Effects of hydration on inner stresses



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Effects of hydration on inner stresses

Hydration

- Thermo-activation (Ulm)
- Hydration heat
- Endogenous shrinkage

 $C\dot{T} = \nabla(K\nabla T) + L\dot{\xi}$

- C specific heat capacity
- *K* thermal conductivity (*Wm*⁻¹*K*⁻¹)
- *L* is the total activation energy (*Jm*⁻³)

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Effects of hydration on inner stresses

Hydration

- Thermo-activation (Ulm)
- Hydration heat
- Endogenous shrinkage

$$\dot{\varepsilon}_{au_{ij}} = -k\dot{\xi}\delta_{ij}$$
 for $\xi > \xi_0$

- ε_{au} autogenous shrinkage
- k shrinkage coefficient
- ξ₀ is the setting value of ξ for which the paste becomes to be elastic

$$\dot{\varepsilon}_{th_{ij}} = \alpha \dot{T} \delta_{ij}$$

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- ε_{th} thermal strain
- α coefficient of expansion

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Coupling



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Coupling

Evolution of the mechanical parameters (from de Schutter)

- Effective coef of hydration $\bar{\xi} = < \frac{\xi \xi_0}{\xi_{\infty} \xi_0} >_+$
- Young's modulus $E(\xi) = E_{\infty} ar{\xi}^{eta}$
- Poisson's ratio $\nu = \nu_{\infty} \sin \frac{\pi * \bar{\xi}}{2} + 0.5 e^{-10 * \bar{\xi}}$
- Tension strenght $f_t(\xi) = f_{t\infty} \bar{\xi}^{\gamma}$
- Fracture energy $G_f = G_\infty ar{\xi}^\gamma$



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Hydration of concrete

Temperature T(K)

degree of hydration ξ

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Hydration of concrete



Macroscopic behavior Young age concrete Scale effects

Hydration of concrete



Along the middle line



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meso-stress σ_{XX}

Macroscopic behavior Young age concrete Scale effects

Hydration of concrete



Along the middle line



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meso-stress σ_{yy}

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Application to scale effects

Dimensions

| Thick | L ₂ | L | D | Notch |
|--------------|----------------|--------------|--------------|------------|
| (<i>m</i>) | (<i>m</i>) | (<i>m</i>) | (<i>m</i>) | Height (m) |
| 0,05 | 1,4 | 1 | 0,4 | 0,5D |

Size of the beam



- Experiments from Rojas, Grégoire & Pijaudier-Cabot
 - Homothety ratio : $1, \frac{1}{2}, \frac{1}{4}, \frac{1}{8}$



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Modeled problem

Grading curves



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Model

Meshing strategy

- Notched beams of different homothety ratios $\frac{1}{2}$; $\frac{1}{4}$; $\frac{1}{8}$.
- Mix meso-macro model



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• 3 different drawings are used for each beam.

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FE Modeling

Meshing strategy

- Nodes do not match.
- Cinematic coupling.



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FE Modeling

Mechanic parameters

| | Young's | Poisson's Tension | | Fracture |
|-------------|-----------|-------------------|-------------|-----------------------|
| | modulus E | Ratio ν | strenght ft | energy G _f |
| | (GPa) | | (MPa) | (J/m^2) |
| Paste | 25 | 0,2 | 3 | 20 |
| Aggregates | 55 | 0,2 | 6 | 60 |
| Homogenized | 39,61 | 0,2 | - | - |

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FE Results



CMOD / load from experiments and simulations

Macroscopic behavior Young age concrete Scale effects

Results



Damage at the peak load for different homothety ratio

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Scale effect law (Bažant, 1976)

Nominal stress :

$$\sigma_n = \frac{3PL}{2eD^2}$$

Nominal notch opening : $U_n = \frac{U}{D}$



Macroscopic behavior Scale effects

Scale effect law (Bažant, 1976)

Intrinsic size :

 $\bar{D} = 0.15637481182436 * D$ Intrinsic nominal stress $\bar{\sigma}_{nu} = 4.35395101484703 * \sigma_{nu}$

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Conclusions

Smooth FE method for mesoscopic scale

- Can be used in 2D or 3D even if 3D needs long computation time
- Take into account the granular compacity
- Useful for couplings
- Easy to use with any model as long as it is the same for aggregates and paste

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