MICRO-MECHANISMS OF CONCRETE FAILURE UNDER CYCLIC COMPRESSION: X-RAY TOMOGRAPHIC IN-SITU OBSERVATIONS

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Abstract: A meso scale compression test of the concrete specimen has been conducted in an in-situ X-ray computed tomography (XCT) facility. Concrete contained coarse aggregates of 5 mm and normal Portland cement. A specimen of $20 \times 20 \times 20$ mm³ size was studied under three cycles in compression. It has been scanned after the completion of each stage in a cyclic (e.g. immediate after loading to the peak and after the unloading) and two dimensional (2D) images of the microstructure were recorded with a resolution of 16.5µm. A great amount of pre-existing damage in the form of micro cracks and voids has been revealed in the scanned images. Further, the micro-mechanisms, such as crack opening and closing, crack tip extension and diversion, crack tip blunting, and elastic recovery of the crack openings and closures were also observed in the images from different cycles. Finally, digital volume correlation (DVC) was utilized to get the macro displacement fields during each load cycle.

1 INTRODUCTION

Quasi-brittle failure of concrete is a very complex phenomenon, due to its complicated multi-phasic microstructure consisting of randomly distributed aggregates, mortar and voids of different shapes, sizes and volume fractions. The macroscopic overall failure of concrete depends on the microscopic properties and localised failure characteristics of constituents. experimental its The investigation of the essentially threedimensional (3D) micro-scale failure has been very challenging, due to the limitations of conventional techniques, which are mostly 2D surface based (e.g., photoelastic methods) or destructive post-mortem analyses.

An increasingly popular technique, capable of scanning the 3D microstructures and observing the micro-scale failure process under progressive loading, is the in-situ X-ray computed tomography (XCT). Its main advantages include the non-destructive nature, multi-length scale resolutions and wide applicability to many solid materials [1]. Most of the existing XCT studies are restricted to reconstruction of 3D the internal microstructures of various materials [2-6] without loading. Recently, some in-situ XCT tests, by integrating a loading rig during the scan were carried out to investigate the internal 3D damage and fracture evolution in some materials, examples Limodin et al. [7] in nodular graphite cast iron, Bay et al [8] in bones, Brault et al. [9] in laminated Mostafavi et. al [10] in composites, and nuclear graphite, and Patterson et. al [11] in aluminium foam.

Limited in-situ XCT tests were also found for concrete-like quasi-brittle materials. For example, Landis and Nagy [12] carried out insitu compressive XCT tests on cylindrical mortar specimens of size 4 mm and investigated the crack initiation and growth in the post-peak regime. Wang et al. [13] established a relation between the overall damage and representative defect area using XCT reconstructed images of asphalt concrete. Wong and Chau [14] related the nucleation of voids to the gray scale value of pixels in normal and high performance concrete under various compressive loadings. Wan and Xue [15] conducted in-situ XCT compression tests on cement and developed relations between the average gray scale value and the damage by comparison of scanned images. Yang and co-workers [16,17,18] have studied the 40mm concrete cubes under in-situ XCT compression tests. They developed the 2D and 3D finite element models based on reconstructed images for fracture modelling of the concrete. Most of these limited number of in-situ XCT tests were focused on the overall estimation of the damage variable, whereas the localised failure characteristics are rarely investigated. In most of them have addition. applied monotonic loading schemes and in-situ XCT tests under cyclic loading have not been reported to the best knowledge of the authors.

In this study, an in-situ micro XCT test of a concrete cube under cyclic compressive loading was carried out, as an attempt for better understanding of the different failure mechanisms using image analysis. Also, recently-developed digital volume correlation (DVC) technique [8,19,20] is utilised to calculate the full 3D deformation fields for the different loading.

2 IN-SITU XCT TEST

A small concrete specimen of size nearly $20 \times 20 \times 20$ mm³ was cut from a $40 \times 40 \times 40$ mm³ concrete cube. The concrete was same as used earlier by authors [17] and made of 5mm aggregate size and ordinary Portland cement. The objective of the small size of the specimen was to get an enhanced resolution and good quality of finite element mesh for the future fracture simulation. In past, the 40 mm size of the specimen was scanned [17] and it was rather difficult to model the same specimen in finite element analysis. Therefore, the specimen size was reduced to enhance the resolution and the total numbers of degrees of freedom of FE mesh. Manchester X-ray imaging facility in the school of materials at University of Manchester, UK, was utilized for the scans. Tungsten target was used with an accelerating voltage of 100kV and 200µA current for all the scans. Flat X-ray attenuations were obtained and a black reference was made before starting the experiment. The projection images were obtained through a silicon flat detector with an exposure time of 1.4s. Nearly, 1500 projection images have been captured by rotating the stage through 360 degree. These radiographs were then converted in to 3D material image using standard filter back reconstruction algorithm. For each scan, the corrections were made to reduce the effect of the artifacts like beam hardening and ring effect. Finally, the satisfactory contrast for reconstructed 2D XCT images was obtained with a resolution of 16.5 µm. 3D images of all scans have been visualized using commercial software [21,22].



Figure 1: a) In-situ experimental setup, b) Load displacement curve.

The contrast in these images was further enhanced by removing the noise using mean and median filters. A complete test setup is shown in the Fig.1a.

Load was applied using a loading stage (Deben rig) with a speed of 0.5mm per min in three cycles with load control. The loaddisplacement curve obtained is shown in Fig.1b. In the first cycle, the specimen was loaded up to 7KN and then unloaded to 50N. Three scans were made during this cycle. First scan was taken at position A (without any load), 2nd scan at position B (at 7KN) and 3rd scan at position C as marked in Fig.1b. In the 2nd cycle, the load was increased up to 10kN and again unloaded to the 50N. 4th and 5th scans were made accordingly at the loaded and unloaded positions (ref. D and E points in Fig.1b). In 3rd cycle, the load was applied up to 24 kN and again unloaded back to the 50kN and consecutive scans were named as F and G respectively as shown in the Fig.1b.

Fracture process under the loading history has been analyzed with respect to overall damage propagation in the specimen during the loading history. The details of the crack propagation from reconstructed XCT images are discussed in the next section.

3 OBSERVATION ON DAMAGE PROPAGATION

Four 2D slices (from each scan A, B, D and F), which represents the same location in the specimen under different loads are used to describe the damage propagation process as shown in Fig.2.

It is clear from the Fig.2a that the specimen pre-existing interfacial, contained some surface and internal cement cracks along with voids. There are many micro pores (referred as small pores in Fig.2) present on the aggregate/cement interface and also within the cement phase. Big voids, which are relatively very large as compared to micro-pores, are also identified in the concrete images (referred as big pores in Fig.2). The initial cracks present in the concrete may be either formed during the hardening process or originated due to the specimen preparation process. Some other researchers have also discussed the presence of the initial cracks in concrete [6,12]. Some of the pre-existing cracks and micro-voids have been closed due to the initial application of compressive load as shown in scan B (refer red circle in Fig.2a&b).



Figure 2: Damage description in reconstructed 2D slices from different loading stages.

It is also clear from Fig.2b that some aggregate readjustments near the boundaries have also taken place in the specimen during the initial loading. The crack closures are increased with further increments of the load. Many cracks parallel to the loading direction have been appeared in the image obtained from the scan F (see Fig.2d). This means that the cracks network has sufficiently developed all over the specimen to reach the failure. The overall crack propagation is a very complex phenomenon and is difficult to understand from 2D images alone. To develop a good understanding of the fracture process, the crack network has been segmented for all the scans. 3D images of the reconstructed cracks network for different loading stages have been shown in Fig.3.

During the reconstruction, a compromise has been made to make the network image clearer for visualization. The tinny individual cracks less than 100 voxels have been filtered out. Fig.3a shows pre-existing 3D network of the cracks and voids in the specimen. A 3D distribution of pre-existing cracks. As one can see the crack is not a flat and smooth surface. It is a combination of many uneven surfaces and sometime connected with very small bridging cracks. One can compare closely Fig.3a&b to observe some crack closures due to initial compressive loading. There are very small changes in the configuration of the cracks network in these images. Hence, the stage can be considered as the part of the elastic loading. Fig.3c has clearly revealed that there is a substantial increase in the micro crack density under 10kN load. Many small cracks have been originated at different places and many cracks have been bridged the preexisting network. This kind of detailed information for the cracks network cannot be concluded from 2D slices alone. Final macro crack surface under load of 24kN has been shown from scan F in Fig.3d. Many new crack surfaces, mostly parallel to the loading direction have bridged to form a wider uneven complex crack network.

view has given the brief description of the



Figure 3: Reconstructed cracks network of specimen.

This section for overall damage can be concluded as follow: some of the pre-existing cracks closed during the initial loading, then the cracks initiated parallel to the loading direction at distinct places, which were bridged to form a macro cracks network due further application of load. Next section describes the localized distribution of strain in the specimens by using the digital volume correlation.

4 DIGITAL VOLUME CORRELATION

Digital volume correlation (DVC) is a 3D extension of the well-known digital image correlation (DIC) technique used in 2D. In DIC, two images (one from the original and other from deformed configurations) were divided into small subsets of areas and then correlated to find out the deformation gradient of these subsets. In the similar way, in DVC the total volume had been divided into small sub-volumes which contain some point of interest. These points of interest had been tracked in the sub-volumes of deformed image. By comparing two sub-volumes from deformed and undeformed configurations, the displacements were interpolated for each subvolume. Before performing the correlation on the volumes, these images were register to reduce the rigid body movement effects. The volumes were manually translated in three directions to obtain the accuracy of nearly one or two pixels for the registration. Final processed images were cropped and saved in raw format. These raw images were used in the DaVis (a Commercial software package

[9]) to correlate the specimen volumes from different cycles. The correlation algorithm is based on the fast Fourier transform. One can refer the pioneer work of Bay et.al [8] for more details on this technique. Three window sizes as 128x128x128, 64x64x64 and 32x32x32 voxels were investigated for the optimum results. Two passes have been made with an overlap of 50% to achieve the results. Fig.4 shows the variation of the z-direction displacement (w) on the center slice of xz plane under 7kN load. Here, z is the loading direction. Figs.4a, b and c show the contours of w for window size 128, 64 and 32 respectively. The displacement contours are smoother as the size of the window is reduced. To show the variation of the *w* displacement in the loading direction (thickness direction), the displacements (w) has been average over each slice in xy plane. Fig.5 shows the variation of the w in the thickness direction of the specimen and the effect of the window size. In Fig.5, w has increased up to the 1/5 th of the specimen height and then linearly decreased toward the bottom. Near the bottom it became as zero and then negative. Similar trend obtained by the three window sizes. The magnitude of the displacement has slightly increased with the reduction in the window The predicted magnitude of size. the displacements by window sizes 64 and 32 are very near to each other. Hence, one can conclude that the 32 is sufficiently large to correlate the displacement gradients and enough small to capture the local variations. Note that all these local variations are not the representations of the micro-strains, these are macroscopically local variations.

These can be treated as the representations of the heterogeneity at the macro level. Due to the heterogeneity and presence of the defects the displacements are increasing first and negative at the bottom of the specimen. One can also notice that the neutral plane has also got shifted towards the middle of specimen due to the application of the load.



Figure 4: Effect of the window size selection on the displacements.



Figure 5: variation of the w displacement along the thickness of the specimen.



Figure 6: variation of the *w* displacement along the thickness of the specimen.

More light on the variation of the deformations can be thrown with the contours of the w-displacements. Fig.6 shows the deformation contours under peak loads of the cycles. There is an increase in the positive displacements at the bottom of the image (ref. Figs.6a and b). whereas the negative displacements i.e movement from the top to bottom of the image is reduced. The contours have also slightly rotated clockwise. With the application of the next loading, displacements are concentrated at the both ends and produced a zero plane nearly at the 45 degrees.

4 CONCLUSIONS

In-situ cyclic compression test has been conducted on a small scale specimen of concrete to explore the damage in the pre-peak region of load displacement curve. It is revealed from the XCT images that specimen contained initial inherent internal defects in addition to surface damage. The local fracture mechanisms such as crack opening and closing during the loading and unloading history have been revealed through the reconstructed XCT images. Elastic recovery of the crack openings and closures has also been observed. 3D image has revealed that macro crack has an irregular shape and contained many micro fronts liable to extension. Also there are many micro cracks originated before the appearance of macro crack. Next, the DVC has been used to find out the localized macro displacements in the specimen. The distribution has revealed the specimen heterogeneity. The displacement contours are highly heterogeneous in the specimen near the peak load. The average w displacements over xy plane are observed high nearly at a distance of 5 mm from the loading edge. This distance is equal to the average distance of the aggregate.

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