Multiscale Modeling of Alkali Silica Reaction Deterioration of Concrete Structures

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- 2 Alkali Silica Reaction (ASR) Model
- 3 The Lattice Discrete Particle Model (LDPM)
- 4 Numerical Simulations
- 5 Interpretation of Nonlinear Ultrasound Measurements
- 6 Mathematical Homogenization

Conclusions



Example of Deterioration : Alkali Silica Reaction (ASR)



Bridges

Nuclear Power Plants







Example of Deterioration : Alkali Silica Reaction (ASR)







Example of Deterioration : Alkali Silica Reaction (ASR)

Dams

Bridges

Nuclear Power Plants





Alkali Silica Reaction (ASR) in a Nutshell

• Chemical Reaction (Very simplistic description)







• Mechanical Deterioration





Introduction







Introduction



















Given

- A specific engineering problem and
- A fine-scale model of reference

A multiscale model is

an **approximation** of the fine-scale solution characterized by the **max accuracy** for a given **acceptable cost** or the **min cost** for a given **required accuracy**

- Accuracy must evaluated against a Calibrated and Validated fine scale model of reference to avoid "Garbage-down, Garbage-up"
- Cost is application and resource dependent.



Available Multiscale Methods

• Information Passing - Discretized subscale material element is embedded into a point of the macro-scale continuum (an integration point of a finite element (e.g. Computational homogenization, mathematical homogenization, microplane model, etc.)

• **Concurrent** - A finite region of the macro-continuum coarse mesh is overlapped or replaced by a fine mesh or discrete sub-structure (meso-structure) model representing the subscale (e.g. Variational multiscale method, bridging scale method, multigrid methods).

• **Coarse-graining** Coarse and fine scale models are both discrete and one particle of the coarse scale simulate the behavior of a certain number of fine-scale particles.



Multiscale Computational Framework

Miniscale

- ASR gel formation
- Water imbibition
- ASR Gel expansion
- Length scale from μm to mm

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- Cracking
- Creep and Shrinkage
- Length scale of mm to cm

Macroscale

- Strength and Stiffnness Degradation
- Length scale of cm to m





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Simplifying a very complex phenomenon

- The aggregate particles are assumed to have spherical shape.
- Silica is assumed to be smeared uniformly over the aggregate volume.
- The dissolution of silica is assumed to progress in a uniform manner in the radial direction inward from the surface towards the particle center.
- The expansion of ASR gel is mostly due to water imbibition.
- Continuous supply of water is needed for the swelling to continue over time.



Reaction front :

$$\dot{z} = -a_s(h,T)w_e(h,\alpha_c)/[r_w c_s z \left(1 - \frac{2z}{D}\right)]$$

ASR Gel permeability :

$$a_s = e^{\left(\frac{E_{ag}}{RT_0} - \frac{E_{ag}}{RT}\right)} a_s^1 \left[1 + \left(\frac{a_s^1}{a_s^0} - 1\right) (1-h)^{n_Z} \right]^{-1}$$

ASR Gel Mass :

$$M_g = \kappa_a \frac{\pi}{6} \left(D^3 - 8z^3 \right) c_s \frac{m_g}{m_s}$$

Effect of Alkali Content

$$\kappa_a = \min(\langle c_a - c_a^0 \rangle / (c_a^1 - c_a^0), 1)$$







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$$A_i = \kappa_i^0 \exp\left(\frac{E_{ai}}{RT_0} - \frac{E_{ai}}{RT}\right) M_g - M_i$$

Imbibition Characteristic time :

 $\tau_i = \delta^2 / C_i$

Water Imbibition Coefficient :

$$C_i = C_i^1 \exp(-\eta M_i) (1 + (C_i^1 / C_i^0 - 1)(1 - h)^{n_M})^{-1}$$

Water Imbibition Rate :

 $\dot{M}_i = A_i / \tau_i$





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LDPM :: Geometry and Kinematics

• Geometry



3D cell



• Discrete compatibility



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Facet Strains

$$\epsilon_{lpha} = rac{1}{r} \llbracket \mathbf{u}_{C}
rbracket \cdot \mathbf{e}_{lpha}; \ \llbracket \mathbf{u}_{C}
rbracket = rac{1}{r} \left(\mathbf{U}^{J} + \mathbf{\Theta}^{J} imes \mathbf{c}^{J} - \mathbf{U}^{I} - \mathbf{\Theta}^{I} imes \mathbf{c}^{I}
ight)$$

LDPM :: Constitutive Equations and Equilibrium

Constitutive Laws

• Fracture and cohesion due to tension and tension-shear

•
$$\varepsilon = \sqrt{\varepsilon_N^2 + \alpha(\varepsilon_M^2 + \varepsilon_L^2)}, t = \sqrt{t_N^2 + (t_M + t_L)^2/\alpha}$$

•
$$t_N = (t/\varepsilon)\varepsilon_N; t_M = \alpha(t/\varepsilon)\varepsilon_M; t_L = \alpha(t/\varepsilon)\varepsilon_L.$$

•
$$\sigma_{bt} = \sigma_0(\omega) \exp\left[-H_0(\omega)\langle \varepsilon - \varepsilon_0(\omega) \rangle / \sigma_0(\omega)\right];$$

• Compaction and pore collapse from compression

•
$$-\sigma_{bc}(\varepsilon_D, \varepsilon_V) \le t_N \le 0$$
; $\sigma_{bc} = \sigma_{c0} + \langle -\varepsilon_V - \varepsilon_{c0} \rangle H_c(r_{DV})$;

Frictional Behavior

•
$$\dot{t}_M = E_T(\dot{\varepsilon}_M - \dot{\varepsilon}_M^p)$$
 $\dot{t}_L = E_T(\dot{\varepsilon}_L - \dot{\varepsilon}_L^p);$
 $\varphi = \sqrt{t_M^2 + t_L^2} - \sigma_{bs}(t_N)$
• $\sigma_{bs} = \sigma_s + (\mu_0 - \mu_\infty)\sigma_{N0}[1 - \exp(t_N/\sigma_{N0})] - \mu_\infty t_N$

Translational and rotational equilibrium equations of each particle

$$M_u^I \ddot{\mathbf{U}}^I - V^I \mathbf{b}^0 = \sum_{\mathcal{F}_I} A \mathbf{t}^{IJ}$$
; $M_{\theta}^I \ddot{\mathbf{\Theta}}^I = \sum_{\mathcal{F}_I} A(\mathbf{c}^I \times \mathbf{t}^{IJ} + \mathbf{m}^{IJ})$





LDPM :: Some Results















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The Lattice Discrete Particle Model (LDPM)

Unconfined Compression, Cont.



Figure: Unconfined compressive behavior. a) Contours of meso-scale crack opening at failure for low friction boundary conditions; b) Contours of meso-scale crack opening at failure for high friction boundary conditions



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Unconfined Compression, Cont.



Figure: a) Low friction coefficient $\mu(s) = \mu_d + (\mu_s - \mu_d)s_0/(s_0 + s)$, $\mu_s = 0.03$, $\mu_d = 0.0084$, and $s_0 = 0.0195$ mm; b) Stress-strain curves for cubes; c) Lateral expansion for cubes; d) Stress-strain curves for very short prisms; e) Stress-strain curves for short prisms; f) Stress-strain curves for long prisms.



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LDPM :: Even More Results

Over Reinforced Beam



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Deep Beam in Shear



Amalgamation of ASR Model with LDPM

- Radius increase of each aggregate particle $r_i = \left(\frac{3M_i}{4\pi\rho_w} + r^3\right)^{1/3} r$
- Normal meso-scale eigenstrain $e^a_N = \langle (r_{i1}+r_{i2})/2 \delta_c \rangle / \ell$ and $\dot{e}^a_N = 0$ for $\zeta = 0$

•
$$e_M^a = e_L^a = 0 \rightarrow \epsilon^a = [e_N^a \quad 0 \quad 0]^T$$

• Incremental LDPM strain

$$\Delta \boldsymbol{\epsilon}^* = \Delta \boldsymbol{\epsilon} - \Delta \boldsymbol{\epsilon}^a$$



Limitations. The formulation does not account explicitly for ...

- ... the actual chemistry of gel formation.
- ... multi ion transport.

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- ... gel expansion into pores and cracks.
- ... non-uniform silica distribution within the aggregate.
- ... local water diffusion through (a) pores, (b) cracks; and (c) gel.



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Model Calibration

Data by S. Multon and F. Toutlemonde, 2006. Cement and Concrete Research, 36:912-920.

Simulated 1 : No Creep nor Shrinkage 0.12 Sealed 0.09 Volumetric Strain [%] 0.06 240 mm 0.03 000* Experimental \cap Simulated -0.03Simulated 2 <u>_ 130 mm</u> Simulated 3 -0.060 90 180 270 360 450 Time [days]



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Validation :: Stress Effect - Axial Stresses





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Validation :: Stress Effect - Axial Stresses





Validation :: Stress Effect - Axial Stresses





• Free Expansion

• 20 MPa / Unrest.

• 00 MPa / Rest.



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Nonlinear Acoustic Nonlinearity Techniques



Acoustic Nonlinearity Parameter (ANLP)

 $\beta = \frac{8A_2}{k^2 L A_1^2} \propto A_2 / A_1^2$ How does it relate to damage?

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Accelerated Mortar Bar Test with ANLP testing





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Alkali Ion Macro Diffusion

Nonlinear Alkali ion diffusion model

$$\frac{dc_i}{dt} = \nabla \cdot \left(D_i(c_i) \nabla c_i \right) \text{ With } D_i(c_i) = D_i^1 \left[1 + \left(\frac{D_i^1}{D_i^0} - 1 \right) \left\langle 1 - \frac{c_i}{c_i^{max}} \right\rangle^{n_i} \right]^{-1}$$

Modification of ASR-LDPM for Variable Alkali Content



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Numerical Simulation of ANLP





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Parametric Analysis



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Separation of Scales



• Separation of scales

 $\mathbf{x} = \eta \mathbf{y}$ $0 < \eta << 1$

- x is the coarse scale coordinate system
- y is the fine scale coordinate system

RVE problem at each macroscopic Gauss point



Asymptotic Expansion



• Asymptotic expansion of particle displacement

$$\mathbf{u}(\mathbf{x}, \mathbf{y}) \approx \mathbf{u}^0(\mathbf{x}, \mathbf{y}) + \eta \mathbf{u}^1(\mathbf{x}, \mathbf{y})$$

• Asymptotic expansion of particle rotation

$$\frac{\boldsymbol{\theta}(\mathbf{x}, \mathbf{y}) \approx \eta^{-1} \boldsymbol{\omega}^{0}(\mathbf{x}, \mathbf{y}) + \boldsymbol{\varphi}^{0}(\mathbf{x}, \mathbf{y}) + \boldsymbol{\omega}^{1}(\mathbf{x}, \mathbf{y}) + \eta \boldsymbol{\varphi}^{1}(\mathbf{x}, \mathbf{y})}{\text{Northwestern}}$$

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Several Pages of Mathematical Manipulations



Multiple Scale Equilibrium Equations





Rigid Body Motion

• Considering facet level elastic constitutive law along with

with
$$\mathcal{O}(\eta^{-2})$$

- $\mathbf{u}^{\mathbf{0}}$ and $\boldsymbol{\omega}^{\mathbf{0}}$ are rigid body motion and rotation of the RVE - $u_i^0(\mathbf{x}, \mathbf{y}) = v_i^0(\mathbf{x}) + \varepsilon_{ijk} y_k \omega_j^0(\mathbf{x})$
- revise facet strain and curvature definition $\epsilon_{\alpha} = \epsilon_{\alpha}^0 + \eta \epsilon_{\alpha}^1$; $\eta \chi_{\alpha} = \psi_{\alpha}^0 + \eta \psi_{\alpha}^1$

$$\begin{split} & \text{Fine-scale displacement jump} \\ & \varphi_{ij}^{0} = v_{j,i}^{0} - \varepsilon_{ijk}\omega_{k}^{0} \\ & \varepsilon_{\alpha}^{0} = \bar{r}^{-1} \left(\overbrace{u_{i}^{1J} - u_{i}^{1I} + \varepsilon_{ijk}\omega_{j}^{1J}\bar{c}_{k}^{J} - \varepsilon_{ijk}\omega_{j}^{1I}\bar{c}_{k}^{I}} \right) e_{\alpha i}^{IJ} + P_{ij}^{\alpha} (\gamma_{ij} + \varepsilon_{jmn}\kappa_{im}y_{n}^{c}) \\ & \psi_{\alpha}^{0} = \bar{r}^{-1} \left(\underbrace{\omega_{i}^{1J} - \omega_{i}^{1I}}_{\text{V}} \right) e_{\alpha i}^{IJ} + P_{ij}^{\alpha}\kappa_{ij} \\ & \downarrow \\ & \text{Fine-scale rotation jump} \\ \end{split}$$

 $P^{\alpha}_{ij} = n^{IJ}_i e^{IJ}_{\alpha j}$ is the projection operator



RVE Problem

- Terms of $\mathcal{O}(\eta^{-1})$ state RVE equilibrium equations
- Force equilibrium equation Moment equilibrium equation

$$\sum_{\mathcal{F}_{I}} A \, t^{0}_{\alpha} \mathbf{e}^{IJ}_{\alpha} = 0$$

$$\sum_{\mathcal{F}_{I}}A\left(\mathbf{c}^{I}\times t_{\alpha}^{0}\mathbf{e}_{\alpha}^{IJ}+m_{\alpha}^{0}\mathbf{e}_{\alpha}^{IJ}\right)=0$$





Macroscopic Governing Equations

- Terms of $\mathcal{O}(\eta^0)$ state macroscopic equilibrium equations
- Macroscopic translational equation of motion

 $\rho_u = \sum_I M_u^I/V_0$ mass density of the macroscopic continuum



Macroscopic Governing Equations

• Terms of $\mathcal{O}(\eta^0)$ state macroscopic equilibrium equations

- Macroscopic rotational equation of motion



ASR and Homogenization :: Models



(Left) Two-scale homogenization problem. (Right) Full mesoscale prism model.





ASR and Homogenization :: Baseline Results



Volumetric expansion of concrete prism for different alkali contents by experiment and full fine-scale analysis.



ASR and Homogenization :: Effect of RVE Size



Volumetric expansion using RVE size of (left) 35 mm (center) 50 mm, and (righ) 75 mm.



ASR and Homogenization :: Effect of RVE Size, Cont



Comparison of the homogenization results using different RVE sizes with the full fine-scale ones.





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Mathematical Homogenization Fi

ASR and Homogenization :: Material Deterioration



Comparison of the compressive strength obtained by the full fine-scale simulation and the homogenization framework.



ASR and Homogenization :: Crack Patterns



Crack opening contour of the concrete prism and RVEs of different sizes under (a) free expansion due to ASR effect with $C_a = 3.9 \text{ kg/m}^3$ and (b) uniaxial compression after ASR expansion.



ASR and Homogenization :: Computational Cost



Computational cost in full fine-scale analysis and homogenization framework with different RVE sizes.



ASR and Homogenization :: What is to Come ...



Full scale dam analysis ...





MARS (Modeling and Analysis of the Response of Structures) is a multipurpose object-oriented computational software for simulating the mechanical response of structural systems subjected to short duration events.

It is based on dynamic explicit algorithms and it implements all the capabilities and versatility of a general finite element code.

http://mars.es3inc.com/









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- Alkali Silica Reaction is a complex multiscale-multiphysics phenomenon affecting the deterioration of concrete structures worldwide.
- Predictive computational models are needed to evaluate existing deteriorated structures.
- A multi scale framework was proposed to simulate ASR deterioration within the LDPM framework.
- LDPM can be extended effectively to account for various aging and deterioration phenomena.
- Classical mathematical homogenezation can be used to upscale LDPM response for durability related structural analysis.
- All LDPM-based algorithms are currently implemented in the commercially available MARS software – www.mars.es3inc.com



Thank You ! g-cusatis@northwestern.edu

Free time with my students





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Conclusions